CITY OF LOS ANGELES 2002 CODE

CODE : GRAVITY LOADS: SEE CALCULATIONS LATERAL LOADS : SEISMIC - ZONE 4 SOIL PROFILE TYPE Sp,  $N_a = 1.0$ ,  $N_a = 1.0$  C = 0.4, C = 0.64WIND VELOCITY - ZONE 70 MPH EXPOSURE C

SOILS REPORT BY: GEOCON INLAND EMPIRE, INC. JOB NO. A8349-06-01 DATED OCTOBER 2005 IS PART OF THESE PLANS, AND SHALL BE KEPT AT THE JOB SITE AT ALL TIMES.

### GENERAL

SPECIFIED WORK.

1. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS, DETAILS AND WORKING CONDITIONS AT THE SITE, ACQUAINT HIMSELF WITH ALL EXISTING

- 2. ALL DISCREPANCIES, OMISSIONS, OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF WORKING DRAWINGS, SPECIFICATIONS AND SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
- 3. ALL WORK SHALL COMPLY WITH THE UNIFORM BUILDING CODE AND ALL OTHER LOCAL CODES AND ORDINANCES INCLUDING O.S.H.A. STANDARDS.
- 4. ANY INSPECTION, SPECIAL OR CONTINUOUS, AS REQUIRED BY GOVERNING AGENCIES OR THESE DOCUMENTS SHALL BE PERFORMED BY A REGISTERED DEPUTY INSPECTOR CURRENTLY LICENSED BY THE GOVERNING AGENCY AND APPROVED BY ARCHITECT AND STRUCTURAL ENGINEER. THE INSPECTOR SHALL BE EMPLOYED BY THE OWNER TO CONTINUOUSLY INSPECT THE
- 5. OBSERVATION VISITS TO THE SITE BY THE ENGINEER'S FIELD REPRESENTATIVE SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF
- 6. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
- 7. SHORING: IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBLITY TO DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORMWORK, AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING THE CONSTRUCTION OF THIS BUILDING. EXCESS LOAD CAPACITY OF SLAB SHALL NOT EXCEED LOADS EQUIVALENT TO THE DESIGN SUPERIMPOSED LOADS LESS CONSTRUCTION DEAD AND LIVE LOADS. DESIGN SUPERIMPOSED LOADS INCLUDE LIVE LOAD, PARTITION LOAD, AND ANY OTHER LOAD NOT IN PLACE AT THE TIME OF SHORING.
- 8. EXCAVATION: THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURE INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
- 9. OTHER TRADES: SEE ARCHITECTURAL, ELECTRICAL, AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF PIPE. VENT, DUCT AND OTHER OPENINGS AND DETAILS NOT SHOWN ON THESE STRUCTURAL DRAWINGS. ALL DIMENSIONS ARE TO BE CHECKED AND VERIFIED WITH THE ARCHITECTURAL
- 10. BACKFILL: BACKFILL AROUND THE EXTERIOR PERIMETER OF WALL SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL SEVEN (7) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEMS UNLESS WALLS ARE ADEQUATELY BRACED. BACKFILL SHALL NOT BE PLACED UNTIL AFTER COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCURS.
- 11. BRACING: TEMPORARY BRACING SHALL BE PROVIDED AS REQUIRED TO HOLD ALL COMPONENTS OF THE STRUCTURE IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.
- 12. SAFETY: THE CONTRACTOR SHALL ADEQUATELY PROTECT HIS WORK, ADJACENT PROPERTY, AND THE PUBLIC, AND BE RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR
- 13. ALL ASTM SPECIFICATIONS NOTED ON THE DRAWINGS SHALL BE AS AMENDED TO DATE.
- 14. SHOP DRAWINGS: SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT, AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS. A SET OF APPROVED SHOP DRAWINGS MUST BE FILLED WITH THE CITY ENGINEER BY THE CONTRACTOR.
- 15. SHOP DRAWING CHECK: THE CONTRACTOR SHALL SUPPLY THE ENGINEER WITH TWO BLUELINE SETS OF SHOP DRAWINGS A MINIMUM OF THREE WEEKS PRIOR TO FABRICATION. THE REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT INFER THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ENGINEER.

# FOUNDATION

1. THE CONTRACTOR SHALL NOTIFY INSPECTOR PRIOR TO STARTING EXCAVATIONS OR ANY GRADING WORK.

ON STRUCTURAL DRAWINGS. 3. ALL EXCAVATIONS FOR FOOTING MUST BE INSPECTED AND APPROVED BY

2. EXTEND ALL EXCAVATIONS FOR FOOTINGS BELOW NATURAL GRADE AS SHOWN

- INSPECTOR AND SOIL ENGINEER PRIOR TO PLACEMENT OF FORMS, REINFORCEMENT AND POURING OF CONCRETE.
- 4. ALL FILL SHALL BE OBSERVED AND TESTED BY THE SOIL ENGINEER DURING THE GRADING AND PLACEMENT PROCESS.
- 5. ALL EXISTING LOOSE FILL, IF ANY, SHALL BE REMOVED AND COMPACTED AS REQUIRED BY SOIL REPORT.
- 6. ALL FILL SHALL BE PLACED IN A MAXIMUM 6" LAYERS AND COMPACTED TO 90% DENSITY TO SUPPORT SLABS AND TO 95% DENSITY TO SUPPORT FOOTINGS OR AS REQUIRED BY SOIL REPORT.
- 7. ALL WATER SHALL BE REMOVED FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING OF CONCRETE.
- 8. CONSTRUCTION JOINTS, INTERSECTIONS AND CORNERS OF CONTINUOUS FOOTINGS SHALL HAVE ADDITIONAL REINFORCEMENT AS SHOWN ON DETAIL

AND WALLS DOES NOT INCLUDE ANY HYDROSTATIC PRESSURE.

- 9. ALL COLUMNS AND WALLS REINFORCING SHALL BE DOWELED INTO THE FOOTINGS WITH THE SAME SIZE BARS AND SPACING AS THE WALL AND
- COLUMNS REINFORCING, UNLESS NOTED OTHERWISE. 10. A PERMANENT DE-WATERING SYSTEM IS REQUIRED. DESIGN OF FOUNDATION

# CONCRETE

- 1. CONCRETE SHALL BE REGULAR WEIGHT WITH HARDROCK AGGREGATES. 2. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150 TYPE II.
- HARDROCK AGGREGATES SHALL CONFORM TO ASTM C-33 WITH PROVEN SHRINKAGE CHARACTERISTIC OF LESS THAN 0.04%.
- 4. MIXING OF CONCRETE SHALL CONFORM TO ASTM C-94.
- 5. CONCRETE TYPE AND STRENGTHS SHALL BE AS FOLLOWS:

		_	_		
LOCATION	TYPE	28 DAY STRENGTH	SLUMP	AGGREGATE SIZE	AIR
EXT. WALKS & CURBS	HARDROCK	3000 PSI	4"	1 1/2"	_
SPREAD FOOTINGS	HARDROCK	4000 PSI	4 1/2"	1 1/2"	-
CONTINUOUS FOOTINGS	HARDROCK	4000 PSI	4 1/2"	1 1/2"	-
WALLS	HARDROCK	4000 PSI	4 "	3/4"	_
COLUMNS	HARDROCK	5000 PSI	4 "	3/4"	-
SUSPENDED	HARDROCK	4000 DSI	420	7 /4" 1"	
SLAB & BEAMS	HARDROCK	4000 PSI	<b>*</b>	3/4" 1"	
MAT FOUNDATION	HARDROCK	4000 PSI	4"	1"	_

- 6. SPECIAL INSPECTION IS REQUIRED FOR ALL CONCRETE WITH A COMPRESSIVE
- STRENGTH EXCEEDING 2500 PSI; EXCEPT FOR SLAB ON GRADE. 7. ONLY ONE TYPE AND STRENGTH OF CONCRETE SHALL BE POURED ON THE JOB AT ONE TIME.
- 8. CONTRACTOR SHALL OBTAIN THE STRUCTURAL ENGINEER'S APPROVAL FOR CONSTRUCTION JOINT LOCATED IN ALL ELEVATED SLABS, BEAMS AND
- 9. ALL REINFORCING STEEL, ANCHOR BOLTS AND SLEEVES SHALL BE PLACED AND SECURED IN POSITION PRIOR TO POURING CONCRETE.
- 10. ALL OPENING FOR ELECTRICAL AND MECHANICAL ELEMENTS SHALL BE FORMED WITH SLEEVES PRIOR TO POURING CONCRETE. CORING SHALL NOT BE ALLOWED WITHOUT STRUCTURAL ENGINEER'S APPROVAL.
- 11. ALL DEBRIS SHALL BE REMOVED FROM FORMS BEFORE POURING CONCRETE. 12. MINIMUM CONCRETE COVER SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
- SLAB ON GRADE FORMED CONCRETE, EXPOSED TO EARTH OR WEATHER FORMED CONCRETE, NOT EXPOSED TO EARTH OR WEATHER BEAMS AND COLUMNS PRIMARY REINFORCING . . . . 2" BEAMS AND COLUMNS STIRRUPS AND TIES . . . . . 1 1/2"

- a. SHOTCRETE SHALL COMPLY WITH L.A. BUILDING CODE, SECTION 1924 b. THE 28 DAYS STRENGTH SHALL BE 4000 PSI
- c. CEMENT: PORTLAND CEMENT SHALL CONFORM TO ASTM C15, TYPE II AGGREGATE: NORMAL WIGHT AGGREGATE AND SHALL CONFORM TO ASTM C33. MAX. AGGREGATE SIZE SHALL BE 3/4"
- 14. CALCIUM CHLORIDE OR ADMIXTURE CONTAINING CHLORIDE SHALL NOT BE USED IN POST-TENSIONING CONCRETE.
- 15. SHORING SHOULD REMAINI IN PLACE FOR 7 DAYS. RESHORING SHOULD PROVIDED ON ELEVATED SLAB FOR ADDITIONAL 14 DAYS AFTER REGULAR SHORING ES REMOVED.

# REINFORCING STEEL

1. A	ALL REINFORCING STEEL SHALL BE AS FO	LLOWS:
o	. NO. 3 BARS AND SMALLER :	ASTM A615, GRADE 40
b	o. NO. 4 BARS AND LARGER :	ASTM A615, GRADE 60
c	REINFORCING STEEL TO BE WELDED:	ASTM A706, GRADE 60
d	I. WELDED WIRE FABRIC :	ASTM A185

- 2. ALL BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIALS LIKELY TO IMPAIR BOND.
- 3. ALL BENDS SHALL BE MADE COLD.
- 4. SPLICING OF BARS SHALL HAVE MINIMUM LAP PER SCHEDULE OF DETAIL 7/S-1. IN ALL CONTINUOUS REINFORCEMENT OF ELEVATED SLAB AND FOOTINGS AND HORIZ. BARS IN MASONRY WALLS, UNLESS NOTED OTHERWISE ON DETAILS.
- 5. ALL VERTICAL REINFORCING ARE CONTINUOUS UNLESS SPLICE IS SHOWN ON DETAILS, SEE DETAIL 11/S-5.
- 6. ALL REINFORCING BARS SHALL BE ACCURATELY AND SECURELY PLACED BEFORE POURING CONCRETE OR APPLYING GROUT.
- 7. ALL WELDING TO BE DONE UNDER CONTINUOUS INSPECTION OF A DEPUTY-INSPECTOR REGISTERED BY THE CITY.
- 8. MINIMUM LAP FOR WELDED WIRE FABRIC SHALL BE ONE AND HALF FULL MESH.

## CONCRETE BLOCK MASONRY MASONRY UNITS SHALL BE MEDIUM-WEIGHT UNITS CONFORMING TO ASTM C-90

GRADE N, WITH AN ULTIMATE COMPRESSIVE STRENGTH SEE SCH.

- 2. MORTAR MIX SHALL BE 1 PART CEMENT, 3 PARTS SAND 1/4 HYDRATED LIMES OR LIME PUTTY BY VOLUME, WATER CONTENT SHALL BE THE MINIMUM REQUIRED FOR WORKING CONSISTENCY. MORTAR SHALL BE TYPE S UNLESS NOTED OTHERWISE. SEE SCH. FOR 28 DAY ULTIMATE STRENGTH OF
- 3. GROUT MIX SHALL BE 1 PART CEMENT, 3 PARTS SAND, 2 PARTS PEA GRAVEL BY VOLUME. SEE SCH. FORTHE 28 DAY ULTIMATE STRENGTH OF THE GROUT.
- 4. ALL MASONRY SHALL BE SOLID GROUTED.
- 5. CONTINUOUS INSPECTION IS REQUIRED, PER C.B.C. 1701.5-7.
- 6. FIRST COURSE OF BLOCKS ABOVE OPENING SHALL BE LINTEL BLOCK UNITS. 7. MAXIMUM GROUT LIFT SHALL NOT EXCEED 4'-0" IN ONE DAY UNLESS

PROPER PROCEDURES OF HIGH LIFT GROUTING OF THE GOVERNING AGENCIES

- ARE FOLLOWED. VIBRATE ALL GROUT LIFTS. 8. MASONRY UNITS SHALL BE CLEAN AND FREE OF ALL SUBSTANCE THAT MAY
- IMPAIR BOND. 9. BOND BEAM TYPE UNITS SHALL BE USED FOR ALL HORIZONTAL REINFORCING
- 10. VERTICAL COLD JOINTS ARE NOT ALLOWED UNLESS NOTED OTHERWISE ON
- THE DRAWINGS. 11. HORIZONTAL CONSTRUCTION JOINTS: WHEN GROUTING STOPPED FOR ONE
- HOUR OR LONGER, THE GROUT POUR MUST BE STOPPED AT 11/2" MINIMUM BELOW THE TOP OF THE UPPER UNIT. 12. CONTRACTOR SHALL ADEQUATELY BRACE ALL MASONRY WALLS UNTIL ROOF
- STRUCTURE IS ERECTED COMPLETELY.
- 13. FOR ANCHOR BOLT EMBEDMENT IN WALL SEE SCHEDULE @ 13/S-5. 14. MASONRY UNIT ULTIMATE STRENGTH SCHEDULE

F'm	28 DAYS ULTIMATE STRENGTH		
r m	GROUT	MORTAR	
1500 PSI	2000 PSI	2000 PSI	
3500 PSI	4000 PSI	4000PSI	

### SPECIAL NOTES TO OWNER

- UNDER NORMAL CONDITIONS, AND FOR CONVENTIONAL BUILDINGS SUCH AS THE SUBJECT MATTER, REINFORCED CONCRETE AS WELL AS POST-TENSIONED CONCRETE DEVELOP CRACKS. THE CRACKS ARE DUE TO INHERENT SHRINKAGE OF CONCRETE, CREEP AND RESTRAINING EFFECTS OF VERTICAL AND OTHER STRUCTURAL ELEMENTS TO WHICH THE BEAMS/SLABS ARE TIED.
- THE CRACKS FORMED ARE NORMALLY COSMETIC. THE SLAB MAINTAINS ITS SERVICEABILITY AND STRENGTH REQUIREMENTS. DUE TO SPECIAL FEATURES OF UNBONDED POST-TENSIONING, IT IS POSSIBLE THAT A NUMBER OF HAIR CRACKS, WHICH WOULD NORMALLY SPREAD OVER A WIDE AREA, WILL INTEGRATE INTO A SINGLE CRACK WITH A WIDTH EXCEEDING 0.01 INCH. IT IS EMPHASIZED THAT ALTHOUGH SPECIAL EFFORT IS MADE TO REDUCE THE POTENTIAL CAUSES AND NUMBER OF SUCH CRACKS, IT IS NOT PRACTICAL TO PROVIDE TOTAL ARTICULATION BETWEEN THE FLOOR SYSTEM AND ITS SUPPORTS AND THEREBY ACHIEVE COMPLETE INHIBITION OF ALL CRACKS.
- MOST SUCH CRACKS DEVELOP OVER THE FIRST THREE YEARS OF THE LIFE OF THE FLOOR SYSTEM. CRACKS WHICH ARE WIDER THAN 0.01 INCH MAY NEED TO BE PRESSURE EPOXIED. REFER TO THE NOTES UNDER "ALLOWANCES".
- 4. THE OBJECT OF THE JOINTS PROVIDED IS TO ALLOW MOVEMENT. MOVEMENTS DUE TO CREEP AND SHRINKAGE MAY BE NOTICEABLE AT JOINTS UP TO TWO YEARS AFTER CONSTRUCTION, BEYOND WHICH MOVEMENTS DUE TO VARIATIONS IN TEMPERATURE WILL

#### ALLOWANCES (IN ADDITION TO STEEL SHOWN)

- REINFORCEMENT ALLOWANCE: THE CONTRACTOR SHALL PROVIDE 1500 POUNDS OF REINFORCEMENT
- FOR ENGINEER TO USE AT ENGINEER'S DISCRETION DURING CONSTRUCTION.
- 2. PRESSURE EPOXY ALLOWANCE: THE CONTRACTOR SHALL ALLOW FOR PRESSURE EPOXY (SIMPSON "ETILV22" INJECTION EPOXY, ASTM C-881 OR EQUAL) INJECTION FOR 200 FEET OF CRAKS WHICH MAY DEVELOP IN THE STRUCTURE DURING THE FIRST THREE YEARS.
- CONTRACTOR SHALL REIMBURSE OWNER FOR UNUSED PORTION OF ALLOWANCES ABOVE.

DRY PACK

DRY PACK SHALL BE NON-SHRINK, NONMETALIC FACTORY PRE-MIXED WITH MINIMUM 6000 PSI COMPRESSIVE STRENGTH AT 28 DAYS. (THE L.A.R.R # REQUIRED.)

### INSPECTION NOTES

REFER TO SECTION 306 OF THE 1997 UNIFORM BUILDING CODE FOR AMPLIFICATION OF THE FOLLOWING REQUIREMENTS:

2.	FOUNDATIONS:	YES	NO	N/A
	A. COMPACTED FILL INCLUDING UTILITY TRENCHES	$\boxtimes$		
	B. VISUAL EXAMINATION & APPROVAL OF ALL FOUNDATION EXCAVATIONS	$\boxtimes$		
	C. CONTINUOUS INSPECTION OF PILE DRIVING AND/OR CAISSONS			$\boxtimes$
3.	CONCRETE:			
	CONTINUOUS INSPECTION & TEST CYLINDERS FOR CONCRETE OVER 2500 PSI	$\boxtimes$		

ALL SPECIAL INSPECTORS MUST SUBMIT FINAL REPORTS.

	CONTINUOUS INSPECTION & TEST CYLINDERS FOR CONCRETE OVER 2500 PSI	$\boxtimes$	
4.	REINFORCING AND PRESTRESSING STEEL:		
	A. PLACING OF REINFORCING B. PLACING OF TENDONS		
	C. SAMPLING & TESTING OF STEEL (MILL REPORTS & IDENTIFICATION OF STEEL)	$\boxtimes$	
	D. CONTINUOUS INSPECTION OF INSTALLATION OF REBAR COUPLERS	$\boxtimes$	
	E. CONTINUOUS INSPECTION DURING STRESSING OF PT TENDONS		$\boxtimes$
	F. FIELD MEASURED ELONGATION AND JACKING FORCE RECORDS		$\boxtimes$
	G. GROUTING OF POST—TENSIONED CONCRETE		$\boxtimes$
	H. POST-TENSION TENDON PROTECTIVE WRAPPING		$\boxtimes$

- 5. MASONRY: A. SAMPLING & TESTING OF MASONRY B. SAMPLING & TESTING OF GROUT & MORTAR C. CONTINUOUS INSPECTION
- 6. INSULATING CONCRETE FILL: TEST & INSPECTIONS
- WELDING: A. ALL STRUCTURAL FIELD WELDING (INCLUDES DECKING) B. NON-DESTRUCTIVE TESTING OF
- MOMENT-RESISTING SPACE FRAMES C. STRUCTURAL LIGHT GAGE METAL FRAMING
- A. HIGH STRENGTH BOLTING B. EXPANSION BOLTS IN CONCRETE OR MASONRY 9. STRUCTURAL STEEL: A. MILL REPORTS & IDENTIFICATION OF STEEL
- B. SAMPLING & TESTING 10. APPROVED FABRICATORS: MUST SUBMIT CERTIFICATE OF COMPLIANCE FOR ALL

OFFSITE FABRICATION SUCH AS STRUCTURAL STEEL,

ENGINEER IS CONTRACTED TO DO SO.

(AFFIDAVIT OF COMPLIANCE)

- GLU-LAMS, PRECAST CONCRETE, ETC. 11. ALL TESTS & INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT INSPECTION AGENCY. JOB SITE VISITS BY THE STRUCTURAL ENGINEER DO NOT CONSTITUTE AND ARE NOT A SUBSTITUTE FOR INSPECTIONS UNLESS THE STRUCTURAL
- NOTE: IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO SEE THAT THESE TESTS AND INSPECTIONS ARE PERFORMED.

Los Angeles Regional Uniform Code Program

Committee I-3: Structural Observation



STRUCTURAL OBSERVATION PROGRAM AND DESIGNATION OF THE STRUCTURAL OBSERVER

PROJECT ADDRESS: 20223-20237 W. Saticoy St. Winnetka, Ca. PERMIT APPL. NO .: Description of Work: 4 Story apt. building over subterranean Parking

Owner: Saticoy Realty Inv., LLC	Architect: <u>JAG</u>	Architecture Engineer: 7	ildin Engineerin
		L OBSERVATION items are required)	
Firm or Individual to be responsi	ble for the Struct	ural Observation:	
Name: Tildin Engineering	Phone	e: ( <i>949</i> ) <i>421-0144</i> Calif. Reg	istration: ¢32261
FOUNDATION	WALL	FRAME	DIAPHRAGM
図 Footing, Stem Walls, Piers	⊠ Concrete	☐ Steel Moment Frame	⊠ Concrete
☑ Mat Foundation	☑ Masonry	☐ Steel Braced Frame	☐ Steel Deck
□ Caisson, Piles, Grade Beams	⊠ Wood	☐ Concrete Moment Frame	<b>⊠</b> Wood
☐ Stepp'g/Retain'g Foundation, Hillside Special Anchors	□ Others:	☐ Masonry Wall Frame	☐ Others:
			<del> </del>

**DECLARATION BY OWNER** 

I, the Owner of the project, declare that the above listed firm or individual is hired by me to be the

☐ Others:

DECLARATION BY ARCHITECT OR ENGINEER OF RECORD (required if the Structural Observer is different from the Architect or Engineer of Record) I, the Architect or Engineer of record for the project, declare that the above listed firm or individual is designated by me to be responsible for the Structural Observation.

GENERAL NOTES FOR STRUCTURAL OBSERVATION 1. Structural observation is required for the structural system in accordance with MGD 110. Structural observation is the visual observation of the elements and connections of the structural system at significant construction stages and the complete structure for general conformance to the approved plans and specifications. Structural observation does not waive the responsibility for the inspections required of the building inspector or the deputy inspector.

2. The owner shall employ a civil or structural engineer or architect to perform the structural observation. The engineer or architect shall be registered or licensed in the State of California. The Department of Building and Safety recommends the use of the engineer or architect responsible for the structural design when they are independent of the contractor.

3. The structural observer shall provide evidence of employment by the owner. A letter from the owner or a copy of the agreement for services shall be sent to the building inspector before the first site visit. The structural observer shall also inform the owner of the requirements for a preconstruction meeting and shall preside over this meeting. 4. The owner or owner's representative shall coordinate and call for a meeting between the engineer or architect responsible for the structural design, structural observer, contractor, affected subcontractors and deputy inspectors. The purpose of the meeting shall

to review scheduling of the required observations. A record of the meeting shall be included in the first observation report submitted to the building inspector. 5. The structural observer shall perform site visits at those steps in the progress of the work that allow for correction of deficiencies without substantial effort or uncovering of the work involved. At a minimum, the following significant construction stages require

be to identify the major structural elements and connections that affect the vertical and lateral load systems of the structure and

Construction Stage

Elements/Connections to be Observed

a) PRIOR TO CONC. POUR AT FOUNDATION | Rebar placement 4 wall | col. dowels.

b) PRIOR TO CONC. POUR AT WALLS | Rebar placement.

a site visit and an observation report form the structural observer:

c) PRIOR TO COINC. POUR AT SUSPENDED SLAB! Rebar Placement.
d) PRIOR TO IMASONRY CROUT
e) AT 100% 2nd, 3rd, 4th & Roof Framing! Beam & dray members, diaph. & shear walls but Prior to covering the structural members. 6. The structural observer shall prepare a report of the Department form B&S 261 for each significant stage of construction observed. The original of the observation report shall be sent to the building inspector's office and shall be signed and sealed (wet stamp) by the responsible structural observer. One copy of the observation report shall be attached to the approved plans. The copy attached to the plans need not be sealed but shall be signed by the responsible structural observer or their designee. Copies of the report

shall also be given to the owner, contractor, and deputy inspector. 7. A final observation report must be submitted which shows that all observed deficiencies were resolved and structural system generally conforms with the approved plans and specifications. The Department of Building and Safety will not accept the structural work without this final observation report and the correction of specific deficiencies noted during normal building and deputy

8. The structural observer shall send the original observation report to the following inspection office:

Inspection Group Name Community of LA, CA, Zip

9. When the owner elects to change the structural observer of record, the owner shall: a) notify the building inspector in writing before the next inspection

ALL DRAWINGS AND WRITTEN MATERIAL APPEARING HEREIN CONSTITUTE THE ORIGINAL AND UNPUBLISHED WORK OF THE ARCHITECT AND THE SAME MAY NOT BE DUPLICATED, USED OR DISCLOSED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT.

b) call an additional preconstruction meeting, and

c) furnish the replacement structural observer with a copy of all previous observation reports. The replacement structural observer shall approve the correction of the original observed deficiencies unless otherwise approved by plan check supervision. The policy of the Department shall be to correct any property noted deficiencies without consideration

10. The engineer or architect of record shall develop all changes relating to the structural systems. The building department shall review and approve al changes to the approved plans and specifications. 11. General contractor shall notify the engineer (structural observer) 48 hour in advance to schedule the site visit.



(949) 752 - 9333FAX: (949) 752-1207

**ENGINEERING** 26501 Rancho Pkwy., Ste. 203 Lake Forest, CA 92630 Telephone: (949) 421-0144 FAX No. : (949) 421-0143

TILDIN JOB No. 2004-61

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DATE: **7-28-06** DRAWN BY: **TEAM** J&G NO. **030703** SCALE: AS NOTED