1. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS, DETAILS AND WORKING CONDITIONS AT THE SITE, ACQUAINT HIMSELF WITH ALL EXISTING

WIND VELOCITY - ZONE TOMPH EXPOSURE C

- SITE CONDITIONS. ALL DISCREPANCIES, OMISSIONS, OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF WORKING DRAWINGS, SPECIFICATIONS AND SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
- 3. ALL WORK SHALL COMPLY WITH THE UNIFORM BUILDING CODE AND ALL OTHER LOCAL CODES AND ORDINANCES INCLUDING O.S.H.A. STANDARDS.
- 4. ANY INSPECTION, SPECIAL OR CONTINUOUS, AS REQUIRED BY GOVERNING AGENCIES OR THESE DOCUMENTS SHALL BE PERFORMED BY A REGISTERED DEPUTY INSPECTOR CURRENTLY LICENSED BY THE GOVERNING AGENCY AND APPROVED BY ARCHITECT AND STRUCTURAL ENGINEER. THE INSPECTOR SHALL BE EMPLOYED BY THE OWNER TO CONTINUOUSLY INSPECT THE
- 5. OBSERVATION VISITS TO THE SITE BY THE ENGINEER'S FIELD REPRESENTATIVE SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF
- 6. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
- 1. SHORING: IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBLITY TO DESIGN AND PROVIDE ADEQUATE SHORING, Bracing, and formwork, as required for the PROTECTION OF LIFE AND PROPERTY DURING THE CONSTRUCTION OF THIS BUILDING. EXCESS LOAD CAPACITY OF SLAB SHALL NOT EXCEED LOADS EQUIVALENT TO THE DESIGN SUPERIMPOSED LOADS LESS CONSTRUCTION DEAD AND LIVE LOADS. DESIGN SUPERIMPOSED LOADS INCLUDE LIVE LOAD, PARTITION LOAD, AND ANY OTHER LOAD NOT IN PLACE AT THE TIME OF SHORING.
- 8. EXCAVATION: THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURE INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES IN ACCORDANCE WITH THE LOCAL BUILDING DEPARTMENT.
- 9. OTHER TRADES: SEE ARCHITECTURAL, ELECTRICAL, AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF PIPE, VENT, DUCT AND OTHER OPENINGS AND DETAILS NOT SHOWN ON THESE STRUCTURAL DRAWINGS. ALL DIMENSIONS ARE TO BE CHECKED AND VERIFIED WITH THE ARCHITECTURAL
- 10. BACKFILL: BACKFILL AROUND THE EXTERIOR PERIMETER OF WALL SHALL NOT BE PLACED UNTIL AFTER THE WALLS ARE SUPPORTED BY THE COMPLETION OF INTERIOR FLOOR SYSTEMS. DO NOT PROCEED WITH BACKFILL UNTIL SEVEN (7) DAYS AS A MINIMUM AFTER THE COMPLETION OF INTERIOR FLOOR SYSTEMS UNLESS WALLS ARE ADEQUATELY BRACED. BACKFILL SHALL NOT BE PLACED UNTIL AFTER COMPLETION AND INSPECTION OF WATERPROOFING WHERE WATERPROOFING OCCURS.
- II. BRACING: TEMPORARY BRACING SHALL BE PROVIDED AS REQUIRED TO HOLD ALL COMPONENTS OF THE STRUCTURE IN PLACE UNTIL FINAL SUPPORT IS SECURELY ANCHORED.
- 12. SAFETY: THE CONTRACTOR SHALL ADEQUATELY PROTECT HIS WORK, ADJACENT PROPERTY, AND THE PUBLIC, AND BE RESPONSIBLE FOR DAMAGE OR INJURY DUE TO HIS ACT OR
- 13. ALL ASTM SPECIFICATIONS NOTED ON THE DRAWINGS SHALL BE AS AMENDED TO DATE.
- 14. SHOP DRAWINGS: SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT, AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS. A SET OF APPROVED SHOP DRAWINGS MUST BE FILLED WITH THE CITY ENGINEER BY THE CONTRACTOR.
- 15. SHOP DRAWING CHECK: THE CONTRACTOR SHALL SUPPLY THE ENGINEER WITH TWO BLUELINE SETS OF SHOP DRAWINGS A MINIMUM OF THREE WEEKS PRIOR TO FABRICATION. THE REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THE REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT NOR DOES IT INFER THAT THEY SUPERSEDE THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ENGINEER.

# STRUCTURAL STEEL

REINFORCING BARS.

- I. ALL STRUCTURAL STEEL SHAPES SHALL BE OF NEW MANUFACTURED, FURNISHED AS ROLLED TO THE SIZE AND LENGTHS SHOWN ON THE DRAWINGS.
- 2. ALL STRUCTURAL STEEL SHALL COMPLY WITH ASTM DESIGNATION AS INDICATED BELOW, U.N.O. ASTM-A992 GRADE 50 a) WIDE FLANGE SHAPES AND PLATES USED FOR BRACED FRAMES AND MOMENT FRAMES-ASTM-A992, GRADE 50 b) ANGELES, CHANNELS, WT SHAPES AND, PLATES USED FOR NON-BRACED FRAMES AND NON-MOMENT FRAMES-ASTM-A36
- d) STRUCTURAL STEEL TUBE SHALL CONFORM TO ASTM A-500, GRADE B 3. A "CITY" LICENSED FRABRICATOR IS REQUIRED FOR STRUCTURAL STEEL

C) PIPE COLUMNS SHALL CONFORM TO ASTM A-53, GRADE B.

- 4. FABRICATION AND ERECTION TO COMPLY WITH AISC SPECIFICATIONS FOR BUILDINGS, AS REVISED TO DATE. REPAIRS TO SHORT-LENGTH SECTIONS
- SHALL BE APPROVED BY THE ENGINEER. 5 PROVIDE HOT DIP GALVANIZE OR 3" MINIMUM CONCRETE COVER AROUND
- ALL STRUCTURAL STEEL BELOW GRADE. 6. ALL BOLTS SHALL BE 3/4" DIAMETER ( ASTM A-307 ), UNLESS SHOWN
- 7. SUBMIT SHOP DRAWINGS FOR REVIEW BY THE ENGINEER PRIOR TO FABRICATION. 8. BOLT HOLES IN STEEL SHALL BE 1/16 INCH LARGER IN DIAMETER THAN
- THE NORMAL SIZE OF BOLT USED, EXCEPT AS NOTED. 9. ALL STRUCTURAL STEEL SHAPES BARS, JOISTS, CONNECTIONS, AND STAIR
- PRIMER 6-203, PITTSBURGH PLATE GLASS COMPANY, OR APPROVED EQUAL. IO. ALL WELDS SHALL BE IN CONFORMITY WITH THE STANDARD CODE FOR ARC AND GAS WELDING OF THE AMERICAN WELDING SOCIETY. ALL STRUCTURAL WELDING SHALL USE THE SHIELDED ELECTRIC ARC PROCESS WITH ETOXX ELECTRODES, USE LOW HYDROGEN ELECTRODES FOR BOLTS AND

ASSEMBLIES TO BE PAINTED WITH ONE SHOP COAT OF INHIBITIVE RED

- II. ALL BUTT OR GROOVE WELDS SHOWN SHALL BE COMPLETE PENETRATION WELDS REQUIRING JOINT PREPARARTION AND/OR BACK UP BARS, UNLESS NOTED OTHERWISE.
- 12. ALL STRUCTURAL SHOP AND FIELD WELDING REQUIRE CONTINUOUS INSPECTION BY A REGISTERED DEPUTY INSPECTOR. SHOP INSPECTION IS NOT REQUIRED FOR A LICENSED FABRICATOR OR FOR A SHOP APPROVED BY THE CITY OR COUNTY OF THE JOB SITE, EXCEPT THAT ALL FULL PENETRATION BUTT WELDS SHALL BE X-RAYED OR ULTRASONICALLY TESTED FOR SOUNDNESS BY A TESTING LABORATORY APPROVED BY THE ENGINEER. ALL STRUCTURAL MELDERS SHALL HAVE BEEN QUALIFIED. FIELD MELDERS SHALL BE QUALIFIED BY THE CITY OR COUNTY OF THE JOB SITE.
- 13. THE USE OF ROLLED STEEL SECTIONS AND/OR BOLTS MANUFACTURED OUTSIDE THE UNITED STATES WILL REQUIRE VERIFICATION THAT THE PRODUCTS COMPLY WITH APPLICABLE ASTM STANDARDS. MILL CERTIFICATES WILL BE REQUIRED FOR ALL STEEL. STEEL GRADES OTHER THAN ASTM-A36 WILL REQUIRE TESTING BY AN APPROVED LABORATORY. ALL FOREIGN BOLTS MUST BE APPROVED BY L.A COUNTY BUILDING AND SAFETY PRIOR TO THEIR USE.

DEPARTMENT.

- ALL LUMBER GRADES SHALL BE DOUGLAS FUR-LARCH ( DF ), UNLESS NOTED OTHERWISE : BEAMS, HEADERS, PURLINS, SUBPURLINS, 4x JOISTS . DF NO. 1 POSTS, TOP PLATES, STUDS . . . . . . . . DF NO. 2 ROOF RAFTERS, FLOOR JOISTS. . . . . . . . DF NO. 2 SILL PLATES - PRESSURE TREATED . . . . . . DF NO. 2
- SILL PLATES SHALL BE A MINIMUM OF 6" ABOVE GRADE OR 3" ABOVE CONCRETE AT EXTERIOR, AND SHALL BE PRESSURE TREATED.
- 3. ALL JOIST HANGERS SHALL BE OF A TYPE APPROVED BY THE BUILDING
- 4. BLOCKING SHALL BE INSTALLED AT THE TOP OF ALL BEARING AND SHEAR
- 5. PLYWOOD TO BE STAMPED BY THE DOUGLAS FIR PLYWOOD ASSOCIATION AND SHALL CONFORM TO U.S. PRODUCT STANDARDS PS 1, WITH EXTERIOR GLUE. PLYWOOD AT EXPOSED EAVES SHALL BE EXTERIOR GRADE PLYWOOD.
- 6. NAILING AT EDGE OF ALL OPENINGS TO BE THE SAME AS BOUNDARY NAILING. ALL NAILS SHALL BE COMMON NAILS EXCEPT WHERE OTHERWISE STATED.

NAIL SPACING IS TO BE AS SHOWN ON THE DRAWINGS.

- ALL BOLT HEADS AND NUTS THAT BEAR AGAINST THE FACE OF WOOD MEMBERS SHALL BE PROVIDED WITH METAL WASHERS. ALL BOLT HOLES
- SHALL BE BORED 1/32" TO 1/16" LARGER THAN BOLT DIAMETER. 8. ALL STUD WALLS SHALL HAVE FIRE BLOCKING AT 8'-O" O.C. MAXIMUM.
- WHERE WOOD IS IN CONTACT WITH CONCRETE OR MASONRY, USE FOUNDATION
- REDWOOD, PRESSURE TREATED DOUGLAS FIR OR SHEET METAL CONNECTORS. 10. PROVIDE MULTIPLE STUDS FOR SOLID BEARING AT ENDS OF MISCELLANEOUS BEAMS WHERE POSTS ARE NOT SHOWN, CARRY STUDS DOWN TO SILL PLATE.
- WITH THE BUILDING CODE AND SHALL BE VERIFIED BY THE ENGINEER 12. PROVIDE 2"x3" CROSS-BRIDGING OR 2" SOLID BLOCKING AT A MAXIMUM OF 8'-0" O.C. FOR FLOOR JOISTS ( CONTACT METAL BRIDGING OR EQUAL MAY

II. HOLES IN STRUCTURAL MEMBERS FOR PIPES AND CONDUIT SHALL COMPLY

- 13. ALL JOISTS, RAFTERS AND HEADERS SHALL HAVE A MINIMUM OF 1 1/2"
- BEARING OVER THE SUPPORTS.
- 14. ALL TOP PLATES SHALL HAVE A 5'-O" MINIMUM LAP SPLICE. 15. THE MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19% ( U.N.O. )

SHALL BE INSPECTED PRIOR TO COVERING.

UNLESS NOTED OTHERWISE.

- 16. PROVIDE IX DIAGONAL LET-IN BRACES OR EQUAL STEEL STRAPS AT EVERY 25 FEET IN ALL STUD WALLS NOT PLYWOOD SHEATHED. BRACING SHALL BE
- CONTINUOUS FROM TOP TO BOTTOM PLATES. "GENERAL NAILING SCHEDULE" SHALL BE UTILIZED FOR NAILING OF DIFFERENT ELEMENTS. NAILING OF VERTICAL AND HORIZONTAL SHEATHING SHALL BE PER SHEAR WALL SCHEDULE AND DIAPHRAGM NAILING REQUIREMENTS RESPECTIVELY. INSTALLATION OF VERTICAL AND HORIZONTAL SHEATHING
- 18. DOUBLE JOISTS SHALL BE PROVIDED UNDER ALL WALLS OR PARTITIONS PARALLEL TO JOISTS. USE SOLID BLOCKING UNDER WALLS OR PARTITIONS PERPENDICULAR TO JOISTS. ALL DOUBLE JOISTS SHALL BE NAILED TOGETHER WITH 16d NAILS AT 9" O.C. STAGGERED, 1 1/2" FROM EDGES
- 19. SILL PLATES FOR EXTERIOR STUD WALLS SHALL BE ATTACHED TO CONCRETE SLAB / FOOTING WITH 1/2" DIA. x 6" LG. ANCHOR BOLTS AT 4'-0" O.C. MAXIMUM & WITHIN 12" OF THE ENDS OF SILL PLATES, UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE. SILL PLATES FOR INTERIOR STUD WALLS SHALL BE ATTACHED TO CONCRETE SLAB / FOOTING WITH 0.177" DIA. x 2 1/4" SHANK LENGTH SHOT PINS ( HILTI OR EQUAL ) WITH METAL WASHERS AT 24" O.C., UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE. A MINIMUM OF TWO
- 20. SPACE ALL NAILS 3/8" MINIMUM FROM PLYWOOD EDGES OR SUPPORTING MEMBER EDGES.
- WHEN LAMINATING VERTICALLY 2-2x MEMBERS TOGETHER, USE 16d NAILS AT
- 22. ENDS OF WOOD MEMBERS ENTERING MASONRY WALLS SHALL HAVE A 1/2" AIR SPACE AROUND TOP, END AND SIDES, UNLESS WOOD IS TREATED WITH APPROVED PRESERVATIVE.
- 23. EACH PLYWOOD SHEET SHALL HAVE A MINIMUM AREA OF 8 SQ. FT. WITH A MINIMUM DIMENSION OF 2 FEET.
- 24. FOR ALL CORNERS AND INTERSECTIONS SEE DETAIL

ANCHORS ARE REQUIRED FOR EACH STUD WALL.

25. FOR NOTCH AND DRILLING OF JOIST, STUDS AND PLATES SEE DETAIL 26. FOR TYPICAL WALL AND SHEAR PANEL FRAMING SEE DETAIL

# PARTIAL NAILING SCHEDULE

	CONNECTION	NAILING
1.	JOIST TO SILL OR GIRDER, TOE NAIL	3-8d
2.	BRIDGING TO JOIST, TOE NAIL EACH END	2-8d
3.	1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE	NAIL 2-8d
4.	WIDER THAN 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL 3-8d	
5.	2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND	FACE NAIL 2-16d
6.	SOLE PLATE TO JOIST OR BLOCKING TYP., FAC SOLE PLATE TO JOIST OR BLOCKING, AT BRAC WALL PANELS	
۲.	TOP PLATE TO STUD, END NAIL	2-16d
8.	STUD TO SOLE PLATE	4-8d TOE NAIL OR 2-16d END NAIL
9.	DOUBLE STUDS, FACE NAIL	16d AT 24" O.C.
10.	DOUBLED TOP PLATES, FACE NAIL DOUBLED TOP PLATES, LAP SPLICE	16d AT 16" O.C. 8-16d
11.	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL 3-8d	
12.	RIM JOIST TO TOP PLATE, TOENAIL	8d @ 6" O/C
13.	TOP PLATES, LAPS AND INTERSECTIONS, FACE N	1AIL 2-16d
14.	CONTINUOUS HEADER, TWO PIECES	16d AT 16" O.C. ALONG EACH EDGE
15.	CEILING JOISTS TO PLATE, TOE NAIL	3-8d
16.	CONTINUOUS HEADER TO STUD, TOE NAIL	4-8d
۱٦.	CEILING JOISTS, LAPS OVER PARTITIONS, FACE I	NAIL 3-16d
18.	CEILING JOISTS TO PARALLEL RAFTERS, FACE N	IAIL 3-16d
19.	RAFTER TO PLATE, TOE NAIL	3-8d
20.	I" BRACE TO EACH STUD & PLATE, FACE NAIL	2-8d
21.	1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL 2-8d	
22.	WIDER THAN 1"X8" SHEATHING TO EACH BEARING, FACE NAIL 3-8d	
23.	BUILT-UP CORNER STUDS	16d AT 24" O.C.
24.	BUILT-UP GIRDER AND BEAM	20d AT 32" O.C. AT TOP \$ BOTTOM \$ STAGGERED 2-20d AT ENDS \$ AT EACH SPLICE
25.	2" PLANKS	2-16d AT EACH BEARING

## GLUED LAMINATED TIMBER

- GLUED LAMINATED BEAMS SHALL BE DOUGLAS FIR-LARCH CONFORMING TO COMBINATION 24F OF THE LATEST AITC IIT SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER AND SHALL BEAR AN AITC STAMP. BEAMS SHALL ALSO CONFORM TO THE LATEST U.B.C. STANDARD NO. 25-10 AND 25-11.
- THE FABRICATOR SHALL FUNISH A CERTIFICATE OF CONTINUOUS INSPECTION BY AN APPROVED TESTING AGENCY STATING THAT BEAMS WERE PRODUCED IN ACCORDANCE WITH THE LATEST AITC IIT SPECIFICATIONS AND THE LATEST PRODUCT STANDARD PS-56. THE FABRICATOR SHALL SUBMIT COMPLETE SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
- 3. BEAMS TO BE CAMBERED AS SHOWN ON THE PLANS, CAMBER SHALL BE ZERO WHEN IT IS NOT SPECIFIED.
- 4. STRESS REQUIREMENTS SHALL BE AS FOLLOWS : ( COMBINATION 24F-V8 FOR CANTILEVER AND CONTINUOUS BEAMS,
- COMBINATION 24F-V4 FOR OTHERS ) FB = 2400 PSI ( MIN. )

6. LAMINATION TO BE 1 1/2", U.N.O.

AN APPROVED PRESERVATIVE.

- FV = 165 PSI (MIN.)
- FC = 650 PSI ( MIN., PERPENDICULAR TO GRAIN ) E = 1800000 PSI ( MIN. )
- 5. GLU-LAM BEAMS ARE FOR DRY CONDITION USE, WITH A MAXIMUM MOISTURE
- CONTENT OF 12%, UNLESS NOTED OTHERWISE.
- 1. CUTTING, NOTCHING OR DRILLING OF GLU-LAM BEAMS SHALL REQUIRE THE APPROVAL OF THE ENGINEER.
- 8. ALL GLU-LAM MEMBERS EXPOSED TO WEATHER SHALL BE PROTECTED WITH
- 9. AN AITC CERTIFICATE OF INSPECTION FOR ALL GLUED LAMINATED TIMBER SHALL BE SUBMITTED TO A BUILDING AND SAFETY DIVISION INSPECTOR PRIOR TO ERECTION.

## PREFABRICATED TRUSS

- 1. THE ROOF/FLOOR TRUSSES DESIGN ARE BASED ON "TJI/PRO" TYPE TRUSSES, BY "TRUS-JOIST MACMILLAN". ANY ALTERNATE EQUAL MUST BE APPROVED BY ENGINEER PRIOR TO BID DATE. ANY COST INCURRED BY THE DESIGNER IN CHANGING FROM THE SPECIFIED PRODUCT TO AN ALTERNATIVE PRODUCT
- SHALL BE BORNE BY THE GENERAL CONTRACTOR. WOOD TRUSSES SHALL BE DESIGNED BY A LICENSED CIVIL AND/OR STRUCTURAL ENGINEER FABRICATION SHALL BE MADE UNDER THE CONTROL OF A CITY APPROVED INSPECTION SERVICE, CONSIDERATION SHALL BE GIVEN FOR CANTILEVER, PARTITIONS AND MECHANICAL EQUIPMENT CONDITIONS.
- 3. INSTALLATION, BRACING, BRIDGING AND BLOCKING OF TRUSSES SHALL BE IN ACCORDANCE WITH MANUFACTURERS DIRECTIONS.
- 4. MANUFACTURER SHALL SUBMIT SHOP DRAWING AND CALCULATION TO ENGINEER
- AND BUILDING DEPARTMENT FOR REVIEW. 5. TRUSS INSTALLER SHALL COORDINATE MECHANICAL EQUIPMENT LOCATIONS AND
- OPENINGS WITH MECHANICAL DRAWINGS. 6. TRUSS DEFLECTION:
  - TOTAL LOAD DEFLECTION...... LESS THAN L/240 LIVE LOAD DEFLECTION..... LESS THAN L/480
- 1. TRUSS SHALL BE DISIGNED FOR THE FOLLOWING LOADS: DEAD LOAD = 18.0 PSF LIVE LOAD = 20.0 PSF
  - DEAD LOAD = 23.0 PSF LIVE LOAD = 40.0 PSF LIVE LOAD = 60.0 PSF @ CANILEYER BALCONIES

PARALLAM BEAM/HEADER

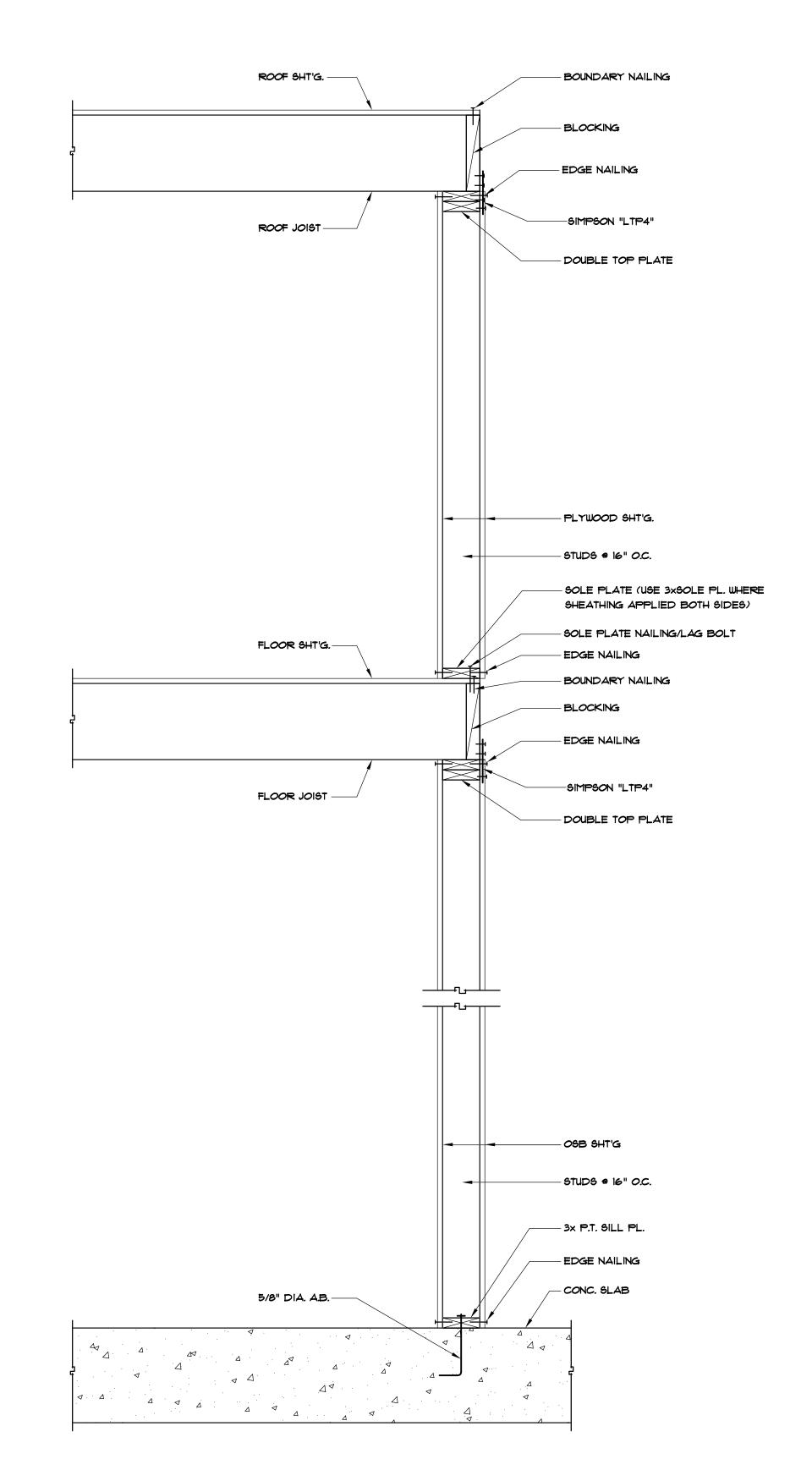
THE PARALLAM BEAMS/HEADERS DESIGN ARE BASED ON 2.0E PARALLAM PSL TYPE BEAM BY "TRUS-JOIST MACMILLAN". SHEAR MODULUS OF ELASTICITY G = 125,000 PSI MODULUS OF ELASTICITY E = 2.0 x 106 PSI

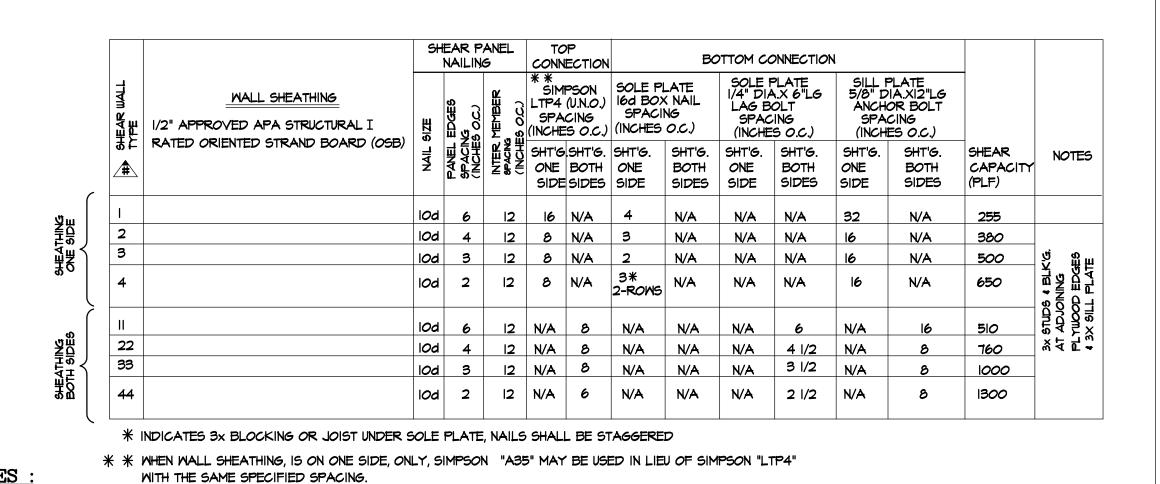
SEE SHEET "SCI.O" FOR STRUCTURAL OBSERVATION PROGRAM 4 NOTES.

- FLEXURAL STRESS F<sub>D</sub> = 2900 PSI COMPRESSION PERPENDICULAR TO GRAIN PARALLEL TO WIDE FACE OF STRANDS F\_ = 650 PSI COMPRESSION PARALLEL TO GRAIN F = 2900 PSI
- TO WIDE FACE OF STRANDS F = 290 PSI

HORIZONTAL SHEAR PERPENDICULAR

# STRUCTURAL OBSERVATION PROGRAM & NOTES:





- NAILING : WHERE POSTS OR MULTIPLE STUDS ARE INDICATED AT THE END OF A SHEAR WALL SUCH MEMBERS SHALL a. COMMON NAILS SHALL BE USED FOR BE EDGE NAILED AND SHALL RUN CONTINUOUSLY FROM THE BOTTOM OR SOLE PLATE TO THE DOUBLE TOP OSB SHEATHING. b. USE MIN. 1/2" EDGE NAILING DISTANCE AT PANEL ENDS AND EDGES. 8. PLYWOOD SPECIFIED AS A SHEAR PANEL SHALL BE
- MAY BE USED IN LIEU OF 1/2" OSB . THE SHEAR WALL ASSEMBLY SHALL RUN HORIZONTALLY AND CONTINUOUSLY TO THE NEAREST OPENING OR

2. 15/32" STRUCT.I PLYWOOD (MIN. 4 PLY)

4. THE SHEAR WALL ASSEMBLY SHALL RUN VERTICALLY AND CONTINUOUSLY FROM THE BOTTOM OF THE NEAREST SOLE OR BOTTOM PLATE UP TO THE TOP OF THE NEAREST DOUBLE TOP PLATES ( OR BEAM ). 5. WHERE A PLYWOOD SHEAR WALL HEIGHT EXCEEDS THE HEIGHT OF A PLYWOOD SHEAR PANEL, PROVIDE SOLID BLOCKING AT UNSUPPORTED PANEL EDGES.

ALL DRAWINGS AND WRITTEN MATERIAL APPEARING HEREIN CONSTITUTE THE ORIGINAL AND UNPUBLISHED WORK OF THE ARCHITECT AND THE SAME MAY NOT BE DUPLICATED, USED OR DISCLOSED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT.

6. EDGE NAILING AT THE END OF A SHEAR WALL (AND SHEAR PANEL) SHALL BE NAILED TO ONE SINGLE STUD OF 1 1/2" MIN. WIDTH, THAT RUN CONTINUOUSLY

TOP PLALTES ( U.N.O. ).

FROM THE BOTTOM OR SOLE PLATE TO THE DOUBLE

OF EACH END AND CORNERS OF WALLS. PROVIDE TWO BOLTS MINIMUM PER WALL. IO. ALL UNSUPPORTED PANEL EDGES SHALL BE BLOCKED WITH 2" NOMINAL OR THICKER FRAMING. (SEE SCHEDULE ABOVE)

9. ANCHOR BOLTS: THE SILL PLATE OF ALL SHEAR WALLS SHALL BE SECURED AS FOLLOWS (U.N.O.):

a. SPACE BOLTS AT 4'-O" O.C. MAXIMUM ( U.N.O. )

( SEE SHEAR WALL SCHEDULE FOR SPACING AT SHEAR WALL ). PROVIDE ONE BOLT WITHIN 12"

A.P.A. STAMPED WITH EXTERIOR GLUE.

THE NAILS ARE SPACED 2" ON CENTER. 2. INTERIOR SHEAR WALLS SHALL RUN CONTINUOUSLY UP TO FLOOR OR ROOF FRAMING ( U.N.O. ). 13. ALL STUDS IN SHEAR WALLS SHALL BE FRAMED AT 16" O.C. MAXIMUM.

ALL NAILS SHALL BE STAGGERED, WHERE

14. PLATE WASHERS SHALL BE USED WITH ALL ANCHOR 5/8" DIA. A.B. = PL. WASHER 2 1/2"x2 1/2"x1/4" 3/4" DIA. A.B. = PL. WASHER 2 3/4"x2 3/4"x5/16" 7/8" DIA. A.B. = PL. WASHER 3"x3"x3/8" 15. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6" ON CENTER ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MENBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED.

7-28-06 DRAWN BY: **TEAM** J&G NO.

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